## SPECIAL MEETING AGENDA MOUNTAIN BROOK CITY COUNCIL

### CITY COUNCIL CHAMBER (A108) 56 CHURCH STREET MOUNTAIN BROOK, AL 35213

### MAY 2, 2023, 5:30 P.M.

As a matter of convenience, members of the public are invited to listen and observe in public meetings by Internet video conference. Presenters and others interested in a particular matter for discussion are required to attend the meeting in-person. The City is not responsible for technical issues that may occur that interfere with the virtual meeting. The City Council, at its sole discretion, may proceed with its in-person business meeting regardless of whether virtual attendees can hear and/or observe the proceedings. The City intends to make the meeting available by way of the Zoom app (re: Meeting ID 801-559-1126, password 05022023).

- 1. Public Discussion: Drainage options for Richmar Drive and Mountain Avenue
- 2. Announcement: The next regular meeting of the City Council is May 8, 2023 at 7:00 p.m.
- 3. Adjourn

## The Cut-Drainage Study

November 20, 2022 Revised: February 9, 2023 Revised: April 4, 2023

The area near the intersection of Richmar Drive and Mountain Lane, locally known as "The Cut", is the focus of this study. For clarity, the study area has been divided into three areas, as described below. Development of the study has utilized recent field survey data, pictures and accounts from affected citizens in the area, and multiple site visits.

The hydrologic modeling used for this study leveraged and extended the previous modeling in the downstream-areas, allowing for the evaluation of downstream effects. This study considers — the planned stormwater improvements in and around the Junior High campus to be complete and in-place.

# Area 1-Mountain Lane and Montevallo Lane Intersection, "The Cut" Open Channel Area

Flood water has been observed in the intersection of Mountain Ln and Montevallo Ln, at the immediate upstream end of The Cut. Overland floodwater has also been observed crossing the front lawn of 26 Montevallo Ln as it flows to The Cut.

Field survey shows a 48" pipe entering from the northeast across Mountain Ln, and at least three more smaller pipes entering The Cut and originating from just south of the intersection.

The drainage area to this point is approximately 77 acres: 56 acres originating in the direction of the 48" pipe, and the remaining 21 acres from south of Mountain Lane.

A lack of surface collection inlets and associated piping is believed to be the primary cause of the excess surface water in this area.

A schematic plan has been developed to capture the surface water on Mountain Ln and Montevallo and direct it through a series of new pipes to the open channel in The Cut. A dependent improvement to the open channel in The Cut will be needed to accommodate the improvements. This work would include clearing the existing vegetation, performing grading, and some minor structural modifications to the headwall of the 48" pipe that drains the channel.

The channel occupies a city alley way that contains available area that could allow the channel volume to be expanded. This additional volume, in combination with some improvements to the headwall of the 48" pipe would provide some increase in overall capacity of the system for the smaller, more frequent storms. However, this is not intended to mitigate the flooding that can result from larger events.

## Estimated Probable Construction Cost \$ 620,000

# Area 2- Richmar Drive and Mountain Lane "The Cut" and Area 3- Open Channel Between Lots on Richmar Drive and Hilldale Road

Prior work in this study identified the 48" pipe in The Cut channel as deficient. The improvement presented included replacing the pipe with a larger one, depending on certain site conditions. These improvements were deemed not viable for the reasons outlined below:

- The receiving channel between the lots on Richmar Dr and Hillsdale Road would require significant upgrades to mitigate the expected increases in peak flow and volume that would occur during a peak event. However, general maintenance to the channel is still recommended to ensure the channel functions at capacity.
- The analysis showed that replacing the pipe at The Cut would result in increases in peak flows downstream of the area, with no reasonable option to mitigate these increases, either by upgrading the primary conveyance system or by creating some type of detention system.

In consideration of these conditions, focus shifted with the goal to manage the potential flood water on Richmar Dr during a peak flood event.

Richmar Dr is lined with valley gutters intended to direct the stormwater south to Montevallo Ln. The lots on the west side of Richmar Dr generally sit at or below the elevation of the road (the east side lots are much higher than street elevation). In the front lawn of several of the lots, there is an elevated area, somewhat like a berm just behind the valley gutter . This increases the depth to which water must rise in the street before it enters the lot. However, near the upper end, a couple of lots do not contain this landscape feature. Also, most driveways contain driveway spanners located in the valley gutter which reduces the water carrying capacity of the gutter. This is especially problematic for the lots that do not contain the natural or built-in protective feature. During a heavy storm event, water can be ejected by the gutter, especially near the spanners, and allowed to flow down gradient into the lots.

One solution is to elevate the area behind the curb in select areas, and for or a couple of the lots, this will include elevating a portion of the driveway.

An alternative option was considered to add a new inlet and pipe network along Richmar Dr that would capture and convey the floodwater south to Montevallo Ln. This would require a significant number of inlets and a long pipe network. The improvements to the lots mentioned previously (to some degree) would also be required, otherwise the potential would still exist for water to enter the lots. A pipe and inlet system like this would be a significant cost and the addition of such would primarily be intended to manage floodwater during peak events.







# Recommended Changes to the City of Mountain Brook Detention Ordinance Executive Summary

## Background

The City of Mountain Brook has several watersheds or drainage basins with aging infrastructure that struggle to safely convey stormwater runoff during large rainfall events. Continued development pressure and additions to existing structures further tax these already strained stormwater drainage systems and exacerbate the drainage problems. As trees and grassed areas are covered by impermeable surfaces, such as rooftops, driveways, and parking lots, more of the rain becomes runoff and leaves the site at a faster rate. To mitigate the adverse effects of development to the existing storm drainage systems and downstream properties, a comprehensive review was conducted to identify and recommend new stormwater regulations.

## **Recommended Modifications to the Detention Ordinance**

Watersheds determined to have existing stormwater infrastructure problems and/or subject to redevelopment pressure are deemed critical watersheds or *critical basins*. The proposed regulations will require a site-specific stormwater design for the proposed development that will manage increases in both stormwater runoff rate and volume to pre-development runoff rate and volume for specified design storm events. As a result, post-construction hydrology should mimic pre-development hydrology within the critical basin.

Specifically, development or redevelopment in critical basins are required to manage the runoff volume from the more frequent 1.1-inch rainfall event as well as ensure post-development peak runoff rates shall be less than or equal to pre-development values for the 2-year through the 100-year, 24-hour rainfall events. The runoff volume from the 1.1-inch rainfall event is termed the *Water Quality Volume (WQ<sub>v</sub>)* and on average 85% of the rainfall events that occur in a given year are 1.1 inches or less. The WQv from the additional impervious area (or an equivalent area of existing impervious area) must be controlled onsite and infiltrated or managed with extended detention. For existing properties that exceed the maximum impervious area (maximum building area plus an additional (5) percent of the parcel area), any new development or improvement on the property will require a stormwater management design to mitigate the entire impervious area that is above the amount maximum allowable.

The stormwater management design shall be performed by a registered architect, landscape architect, or engineer. A written analysis or drainage report supported with design calculations shall be submitted along with plans and details to verify the adequacy of the stormwater management design for the property.

# Table 1 – Critical Basin Summary

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Watershed	Subbasin	Development Pressure	Infrastructure Problems	Severity	Critical Basin (Y/N)
Shades Creek	Furnace Branch	Average	Average	Low	N
	Crestline Branch (West				
	Montcrest)	High	High	High	Y .
	Glencoe Trib	Low	Low	Low	N
	Montclair Branch	High	High	High	Y
	Mountain Brook Club Trib	Average	Average	Low	N
	Mountain Brook Jr High Trib	High	High	High	Y
	Woodhill Road	Low	High	High	Y
Watkins Brook (Shades Creek)	Western Montclair Branch	Average	Average	Low	N
	Eastern Montclair Branch	Low	Average	High	Y
	Southern Residential Trib	High	High	High	Y
	CCB Maintenance Facility				
	Trib	Average	High	High	Y
	English Village Trib	Average	High	High	Y
	Botanical Gardens Trib	Average	High	High	Y
	Heathermoor Trib	High	High	High	Y
	Mountain Brook Mall Trib	High	High	High	Y
	Office Park Trib	High	High	High	Y
Cahaba River	Fuller Creek Upper Reach	Average	High	High	Y
	Fuller Creek Middle Reach	Average	Low	Low	N
	Mt. Brook Presb Trib	Average	Low	Low	N
	Fuller Creek Lower Reach	Average	Low	Low	N
	Overton Branch Eastern Trib	Low	Low	Low	N
	Overton Branch Central Trib	Low	Low	Low	N
	Overton Branch Western				
	Trib	Low	Low	Low	N
	Dunbarton Trib 1	Low	Low	Low	N
	Dunbarton Trib 2	Low	Low	Low	N
	Brookwood Baptist Trib	Low	High	High	Y
Little Shades	Little Shades Creek Upper Reach	Average	Low	Low	N
Creek Basin (Cahaba River)	Little Shades Creek Middle Reach	Average	Low	Low	N

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